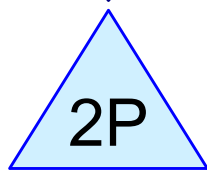
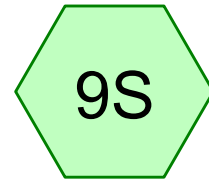


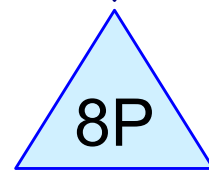
DA



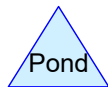
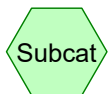
1



DA



1



**Routing Diagram for 5702-031623**

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**5702-031623**

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Page 2

**Area Listing (all nodes)**

Area (acres)	CN	Description (subcatchment-numbers)
124.630	65	2 acre lots, 12% imp, HSG B (1S, 9S)
<b>124.630</b>	<b>65</b>	<b>TOTAL AREA</b>

**5702-031623**

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417 MAIN STREET

NOAA-395-MTP 24-hr S1 2-yr Rainfall=3.58"

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

**Subcatchment 1S: DA** Runoff Area=62.315 ac 12.00% Impervious Runoff Depth>0.66"  
Flow Length=1,760' Slope=0.0860 '/' Tc=35.4 min CN=65 Runoff=22.45 cfs 3.432 af

**Subcatchment 9S: DA** Runoff Area=2,714,429 sf 12.00% Impervious Runoff Depth>0.66"  
Flow Length=1,760' Slope=0.0860 '/' Tc=35.4 min CN=65 Runoff=22.45 cfs 3.432 af

**Pond 2P: 1** Peak Elev=349.68' Storage=143,611 cf Inflow=22.45 cfs 3.432 af  
Primary=0.33 cfs 0.132 af Secondary=0.00 cfs 0.000 af Outflow=0.33 cfs 0.132 af

**Pond 8P: 1** Peak Elev=349.66' Storage=136,053 cf Inflow=22.45 cfs 3.432 af  
Primary=0.30 cfs 0.125 af Secondary=0.45 cfs 0.181 af Outflow=0.76 cfs 0.306 af

**Total Runoff Area = 124.630 ac Runoff Volume = 6.864 af Average Runoff Depth = 0.66"**  
**88.00% Pervious = 109.674 ac 12.00% Impervious = 14.956 ac**

5702-031623

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417 MAIN STREET  
NOAA-395-MTP 24-hr S1 2-yr Rainfall=3.58"

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### Summary for Subcatchment 1S: DA

Runoff = 22.45 cfs @ 12.50 hrs, Volume= 3.432 af, Depth> 0.66"  
Routed to Pond 2P : 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs  
NOAA-395-MTP 24-hr S1 2-yr Rainfall=3.58"

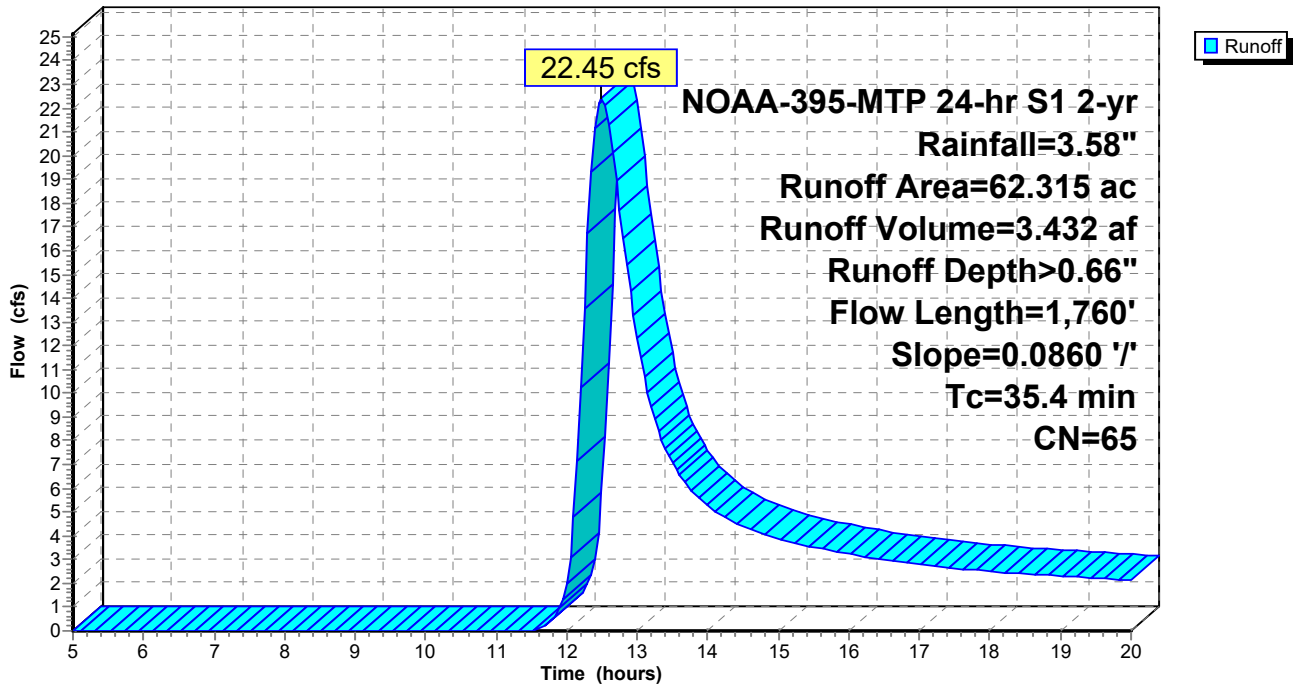
Area (ac)	CN	Description
62.315	65	2 acre lots, 12% imp, HSG B
54.837		88.00% Pervious Area
7.478		12.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
17.1	150	0.0860	0.15		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.00"
18.3	1,610	0.0860	1.47		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
35.4	1,760	Total			

### Subcatchment 1S: DA

Hydrograph



5702-031623

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417 MAIN STREET

NOAA-395-MTP 24-hr S1 2-yr Rainfall=3.58"

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### Summary for Subcatchment 9S: DA

Runoff = 22.45 cfs @ 12.50 hrs, Volume= 3.432 af, Depth> 0.66"  
Routed to Pond 8P : 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs  
NOAA-395-MTP 24-hr S1 2-yr Rainfall=3.58"

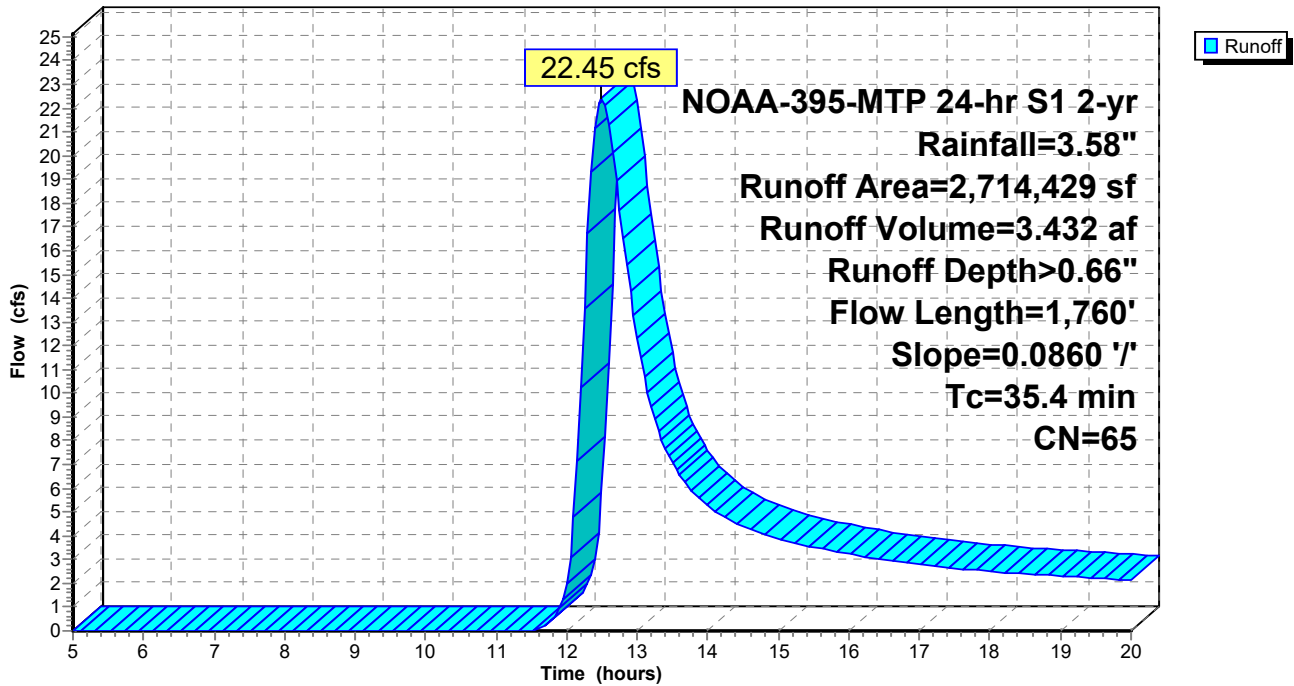
Area (sf)	CN	Description
2,714,429	65	2 acre lots, 12% imp, HSG B
2,388,698		88.00% Pervious Area
325,731		12.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
17.1	150	0.0860	0.15		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.00"
18.3	1,610	0.0860	1.47		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
35.4	1,760	Total			

### Subcatchment 9S: DA

Hydrograph



**Summary for Pond 2P: 1**

Inflow Area = 62.315 ac, 12.00% Impervious, Inflow Depth > 0.66" for 2-yr event  
 Inflow = 22.45 cfs @ 12.50 hrs, Volume= 3.432 af  
 Outflow = 0.33 cfs @ 20.00 hrs, Volume= 0.132 af, Atten= 99%, Lag= 450.1 min  
 Primary = 0.33 cfs @ 20.00 hrs, Volume= 0.132 af  
 Secondary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af  
 Routed to nonexistent node 10R

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs  
 Peak Elev= 349.68' @ 20.00 hrs Surf.Area= 384,964 sf Storage= 143,611 cf

Plug-Flow detention time= 299.7 min calculated for 0.131 af (4% of inflow)  
 Center-of-Mass det. time= 157.3 min ( 1,026.6 - 869.2 )

Volume	Invert	Avail.Storage	Storage Description		
#1	349.30'	787,779 cf	<b>Custom Stage Data (Irregular)</b> Listed below (Recalc)		
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
349.30	379,814	4,396.0	0	0	379,814
350.90	401,993	4,471.0	625,362	625,362	433,208
352.10	40	160.0	162,417	787,779	2,021,915

Device	Routing	Invert	Outlet Devices	
#1	Primary	349.30'	<b>6.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads	
#2	Secondary	350.60'	<b>10.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads	
#3	Primary	352.00'	<b>48.0" W x 2.0" H Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads	

**Primary OutFlow** Max=0.33 cfs @ 20.00 hrs HW=349.68' (Free Discharge)

↑1=Orifice/Grate (Orifice Controls 0.33 cfs @ 2.09 fps)

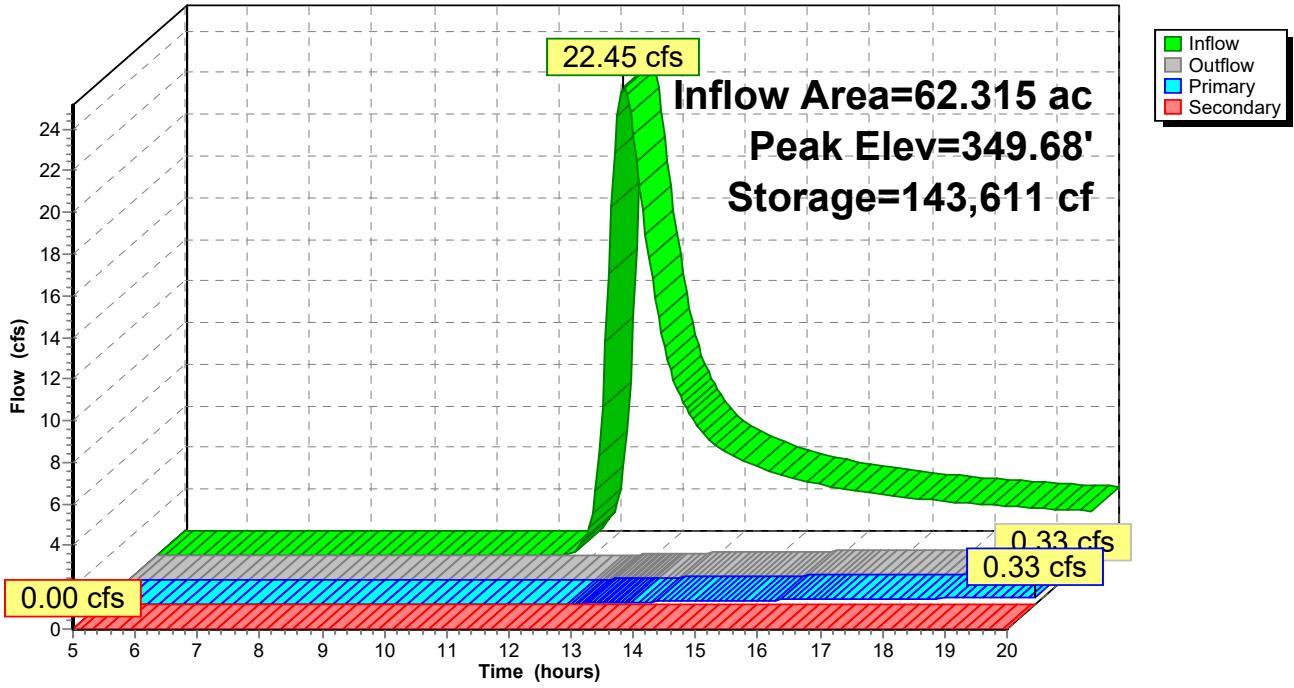
└3=Orifice/Grate ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.00 cfs @ 5.00 hrs HW=349.30' (Free Discharge)

↑2=Orifice/Grate ( Controls 0.00 cfs)

### Pond 2P: 1

Hydrograph



**Summary for Pond 8P: 1**

Inflow Area = 62.315 ac, 12.00% Impervious, Inflow Depth > 0.66" for 2-yr event  
 Inflow = 22.45 cfs @ 12.50 hrs, Volume= 3.432 af  
 Outflow = 0.76 cfs @ 20.00 hrs, Volume= 0.306 af, Atten= 97%, Lag= 450.1 min  
 Primary = 0.30 cfs @ 20.00 hrs, Volume= 0.125 af  
 Secondary = 0.45 cfs @ 20.00 hrs, Volume= 0.181 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs  
 Peak Elev= 349.66' @ 20.00 hrs Surf.Area= 384,693 sf Storage= 136,053 cf

Plug-Flow detention time= 292.7 min calculated for 0.306 af (9% of inflow)  
 Center-of-Mass det. time= 156.4 min ( 1,025.6 - 869.2 )

Volume	Invert	Avail.Storage	Storage Description		
#1	349.30'	625,362 cf	<b>Custom Stage Data (Irregular)</b> Listed below (Recalc)		
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
349.30	379,814	4,396.0	0	0	379,814
350.90	401,993	4,471.0	625,362	625,362	433,208

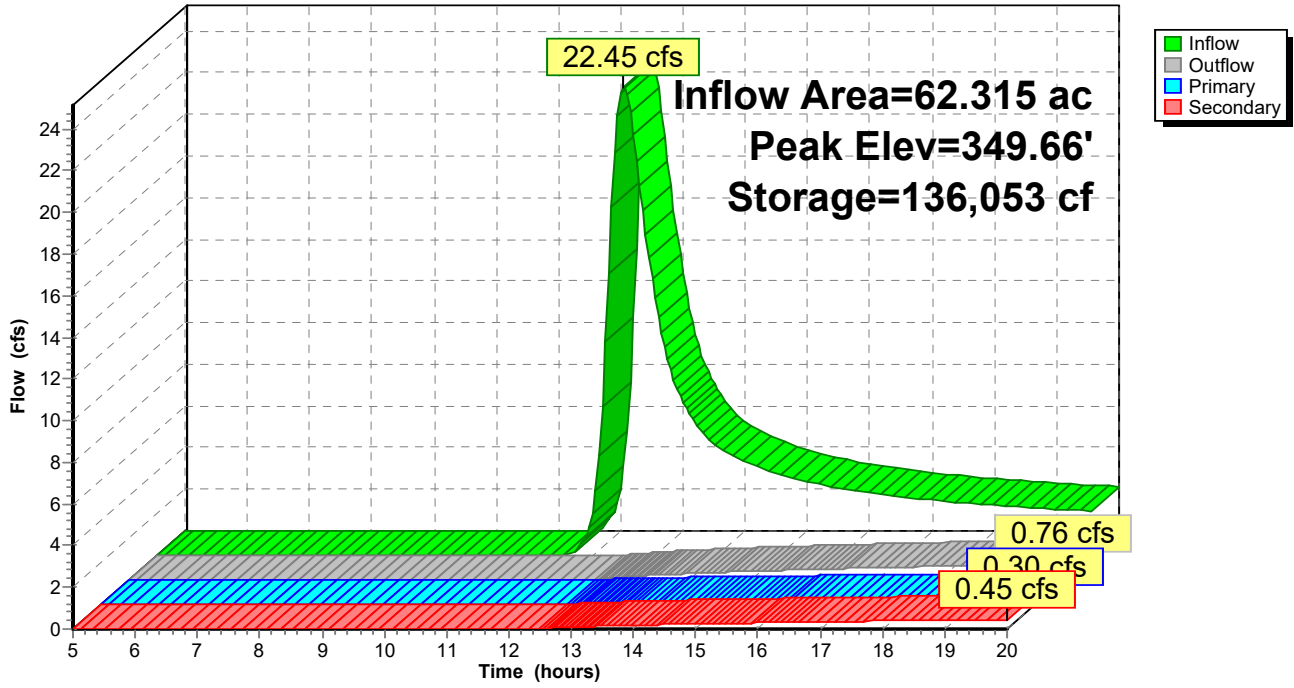
Device	Routing	Invert	Outlet Devices	
#1	Primary	349.30'	<b>6.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads	
#2	Secondary	348.60'	<b>24.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads	
#3	Device 2	349.30'	<b>10.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads	
#4	Device 2	352.50'	<b>4.0" x 4.0" Horiz. Orifice/Grate X 9.00 columns</b> X 9 rows C= 0.600 in 42.0" x 42.0" Grate (73% open area) Limited to weir flow at low heads	
#5	Device 2	351.30'	<b>10.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)	

**Primary OutFlow** Max=0.30 cfs @ 20.00 hrs HW=349.66' (Free Discharge)  
 ↳ **1=Orifice/Grate** (Orifice Controls 0.30 cfs @ 2.03 fps)

**Secondary OutFlow** Max=0.45 cfs @ 20.00 hrs HW=349.66' (Free Discharge)  
 ↳ **2=Orifice/Grate** (Passes 0.45 cfs of 5.89 cfs potential flow)  
 ↳ **3=Orifice/Grate** (Orifice Controls 0.45 cfs @ 2.03 fps)  
 ↳ **4=Orifice/Grate** ( Controls 0.00 cfs)  
 ↳ **5=Sharp-Crested Rectangular Weir** ( Controls 0.00 cfs)

### Pond 8P: 1

Hydrograph



**5702-031623**

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417 MAIN STREET

NOAA-395-MTP 24-hr S1 25-yr Rainfall=6.76"

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

**Subcatchment 1S: DA** Runoff Area=62.315 ac 12.00% Impervious Runoff Depth>2.52"  
Flow Length=1,760' Slope=0.0860 '/' Tc=35.4 min CN=65 Runoff=90.13 cfs 13.096 af

**Subcatchment 9S: DA** Runoff Area=2,714,429 sf 12.00% Impervious Runoff Depth>2.52"  
Flow Length=1,760' Slope=0.0860 '/' Tc=35.4 min CN=65 Runoff=90.12 cfs 13.096 af

**Pond 2P: 1** Peak Elev=350.70' Storage=545,741 cf Inflow=90.13 cfs 13.096 af  
Primary=1.01 cfs 0.554 af Secondary=0.04 cfs 0.002 af Outflow=1.06 cfs 0.557 af

**Pond 8P: 1** Peak Elev=350.56' Storage=490,084 cf Inflow=90.12 cfs 13.096 af  
Primary=0.95 cfs 0.534 af Secondary=2.41 cfs 1.305 af Outflow=3.36 cfs 1.839 af

**Total Runoff Area = 124.630 ac Runoff Volume = 26.192 af Average Runoff Depth = 2.52"**  
**88.00% Pervious = 109.674 ac 12.00% Impervious = 14.956 ac**

5702-031623

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417 MAIN STREET  
NOAA-395-MTP 24-hr S1 25-yr Rainfall=6.76"

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### Summary for Subcatchment 1S: DA

Runoff = 90.13 cfs @ 12.44 hrs, Volume= 13.096 af, Depth> 2.52"  
Routed to Pond 2P : 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs  
NOAA-395-MTP 24-hr S1 25-yr Rainfall=6.76"

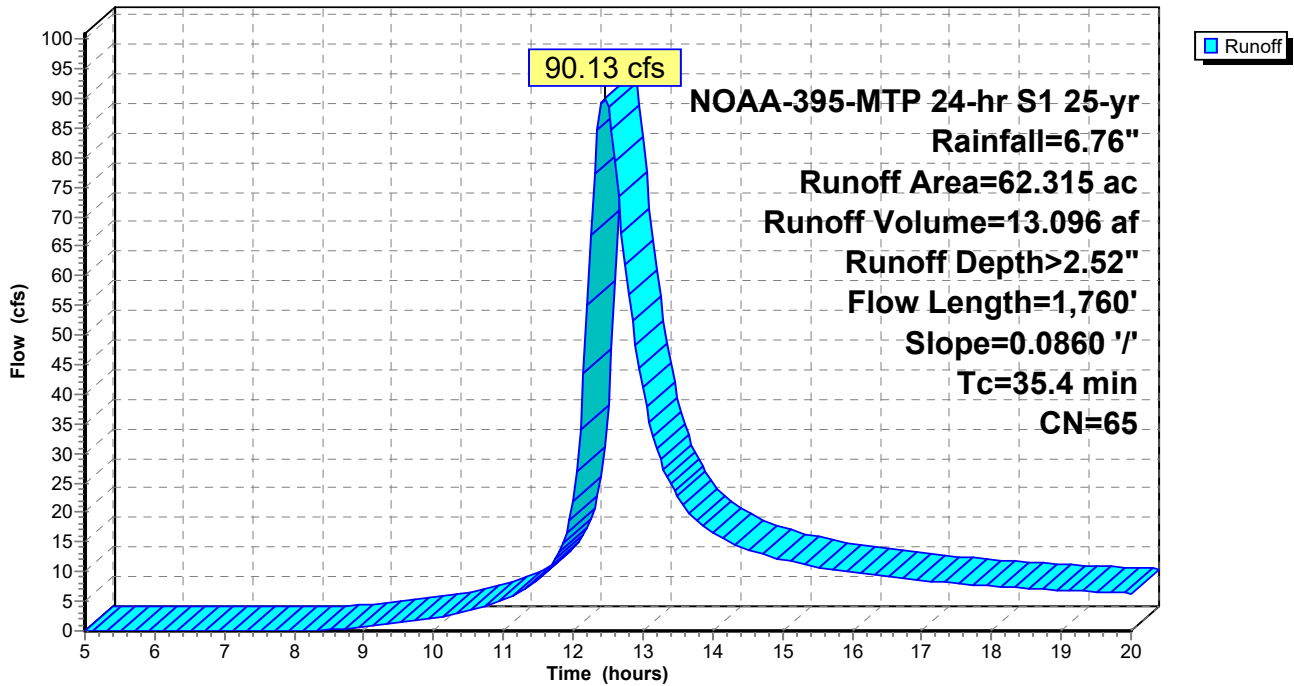
Area (ac)	CN	Description
62.315	65	2 acre lots, 12% imp, HSG B
54.837		88.00% Pervious Area
7.478		12.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
17.1	150	0.0860	0.15		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.00"
18.3	1,610	0.0860	1.47		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
35.4	1,760	Total			

### Subcatchment 1S: DA

Hydrograph



5702-031623

Prepared by J. EDWARDS & ASSOCIATES

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417 MAIN STREET

NOAA-395-MTP 24-hr S1 25-yr Rainfall=6.76"

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### Summary for Subcatchment 9S: DA

Runoff = 90.12 cfs @ 12.44 hrs, Volume= 13.096 af, Depth> 2.52"  
Routed to Pond 8P : 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs  
NOAA-395-MTP 24-hr S1 25-yr Rainfall=6.76"

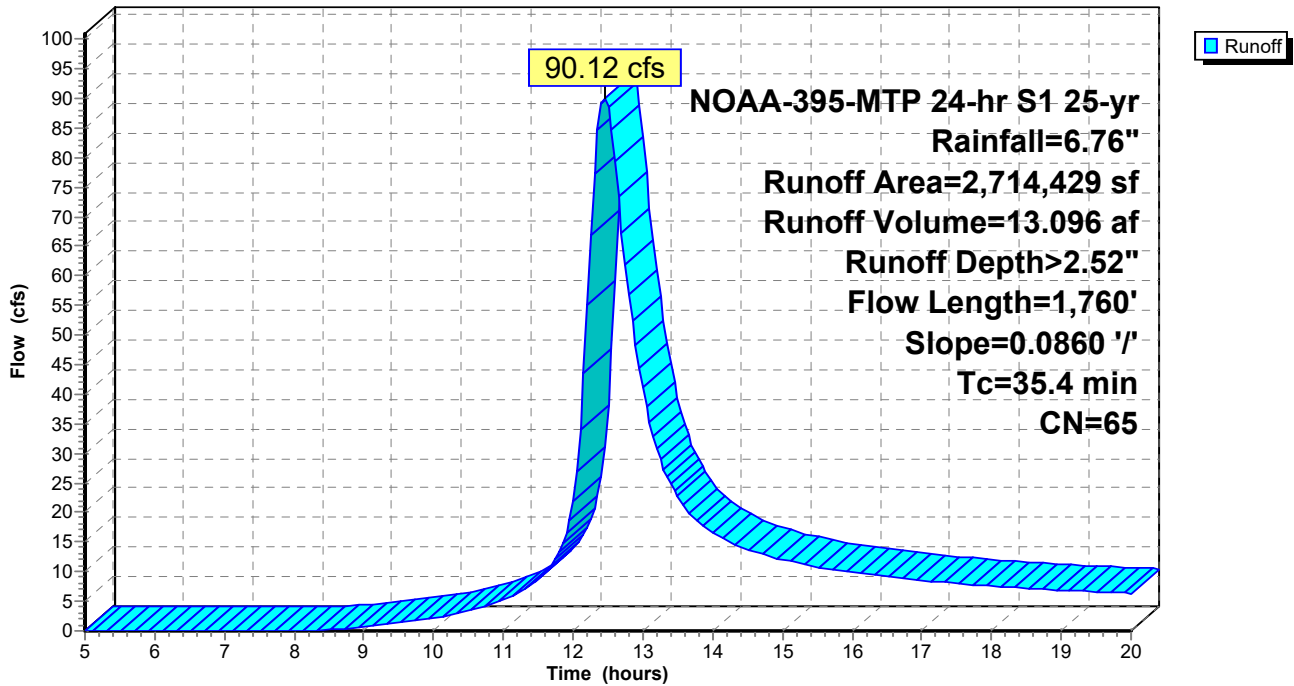
Area (sf)	CN	Description
2,714,429	65	2 acre lots, 12% imp, HSG B
2,388,698		88.00% Pervious Area
325,731		12.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
17.1	150	0.0860	0.15		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.00"
18.3	1,610	0.0860	1.47		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
35.4	1,760	Total			

### Subcatchment 9S: DA

Hydrograph



**Summary for Pond 2P: 1**

Inflow Area = 62.315 ac, 12.00% Impervious, Inflow Depth > 2.52" for 25-yr event  
 Inflow = 90.13 cfs @ 12.44 hrs, Volume= 13.096 af  
 Outflow = 1.06 cfs @ 20.00 hrs, Volume= 0.557 af, Atten= 99%, Lag= 453.3 min  
 Primary = 1.01 cfs @ 20.00 hrs, Volume= 0.554 af  
 Secondary = 0.04 cfs @ 20.00 hrs, Volume= 0.002 af  
 Routed to nonexistent node 10R

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs  
 Peak Elev= 350.70' @ 20.00 hrs Surf.Area= 399,204 sf Storage= 545,741 cf

Plug-Flow detention time= 360.8 min calculated for 0.557 af (4% of inflow)  
 Center-of-Mass det. time= 151.3 min ( 988.7 - 837.4 )

Volume	Invert	Avail.Storage	Storage Description		
#1	349.30'	787,779 cf	<b>Custom Stage Data (Irregular)</b> Listed below (Recalc)		
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
349.30	379,814	4,396.0	0	0	379,814
350.90	401,993	4,471.0	625,362	625,362	433,208
352.10	40	160.0	162,417	787,779	2,021,915

Device	Routing	Invert	Outlet Devices	
#1	Primary	349.30'	<b>6.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads	
#2	Secondary	350.60'	<b>10.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads	
#3	Primary	352.00'	<b>48.0" W x 2.0" H Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads	

**Primary OutFlow** Max=1.01 cfs @ 20.00 hrs HW=350.70' (Free Discharge)

↑1=Orifice/Grate (Orifice Controls 1.01 cfs @ 5.17 fps)

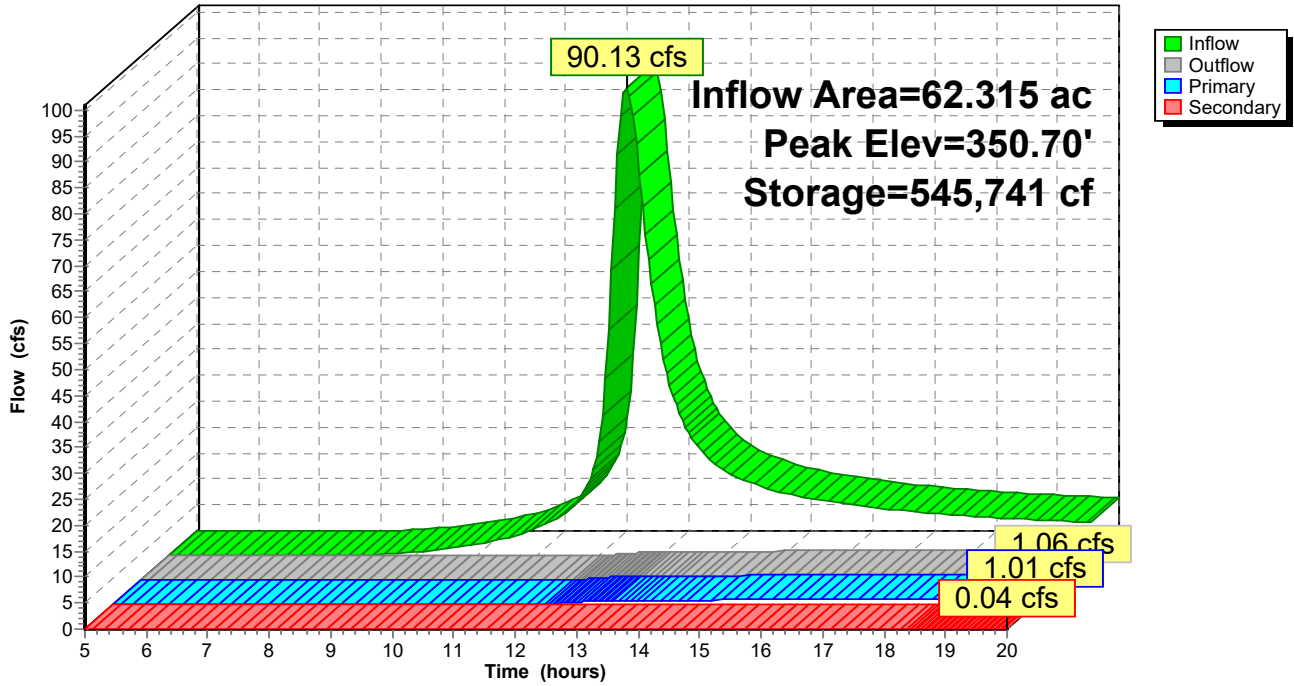
└3=Orifice/Grate ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.04 cfs @ 20.00 hrs HW=350.70' (Free Discharge)

↑2=Orifice/Grate (Orifice Controls 0.04 cfs @ 1.08 fps)

### Pond 2P: 1

Hydrograph



**Summary for Pond 8P: 1**

Inflow Area = 62.315 ac, 12.00% Impervious, Inflow Depth > 2.52" for 25-yr event  
 Inflow = 90.12 cfs @ 12.44 hrs, Volume= 13.096 af  
 Outflow = 3.36 cfs @ 20.00 hrs, Volume= 1.839 af, Atten= 96%, Lag= 453.3 min  
 Primary = 0.95 cfs @ 20.00 hrs, Volume= 0.534 af  
 Secondary = 2.41 cfs @ 20.00 hrs, Volume= 1.305 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs  
 Peak Elev= 350.56' @ 20.00 hrs Surf.Area= 397,248 sf Storage= 490,084 cf

Plug-Flow detention time= 301.6 min calculated for 1.833 af (14% of inflow)  
 Center-of-Mass det. time= 152.8 min ( 990.1 - 837.4 )

Volume	Invert	Avail.Storage	Storage Description		
#1	349.30'	625,362 cf	<b>Custom Stage Data (Irregular)</b> Listed below (Recalc)		
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
349.30	379,814	4,396.0	0	0	379,814
350.90	401,993	4,471.0	625,362	625,362	433,208

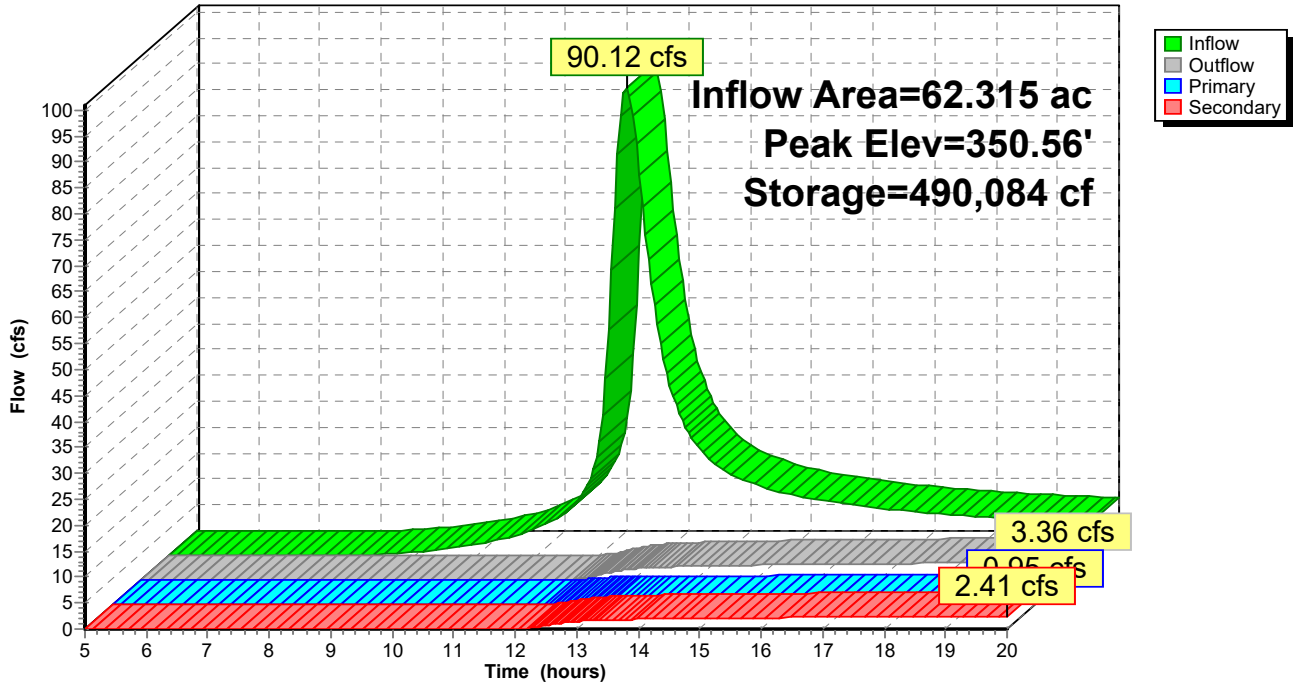
Device	Routing	Invert	Outlet Devices	
#1	Primary	349.30'	<b>6.0" Vert. Orifice/Grate</b>	C= 0.600 Limited to weir flow at low heads
#2	Secondary	348.60'	<b>24.0" Vert. Orifice/Grate</b>	C= 0.600 Limited to weir flow at low heads
#3	Device 2	349.30'	<b>10.0" Vert. Orifice/Grate</b>	C= 0.600 Limited to weir flow at low heads
#4	Device 2	352.50'	<b>4.0" x 4.0" Horiz. Orifice/Grate X 9.00 columns</b>	X 9 rows C= 0.600 in 42.0" x 42.0" Grate (73% open area) Limited to weir flow at low heads
#5	Device 2	351.30'	<b>10.0' long Sharp-Crested Rectangular Weir</b> 2 End Contraction(s)	

**Primary OutFlow** Max=0.95 cfs @ 20.00 hrs HW=350.56' (Free Discharge)  
 ↑1=Orifice/Grate (Orifice Controls 0.95 cfs @ 4.84 fps)

**Secondary OutFlow** Max=2.41 cfs @ 20.00 hrs HW=350.56' (Free Discharge)  
 ↑2=Orifice/Grate (Passes 2.41 cfs of 14.91 cfs potential flow)  
 ↑3=Orifice/Grate (Orifice Controls 2.41 cfs @ 4.43 fps)  
 ↑4=Orifice/Grate ( Controls 0.00 cfs)  
 ↑5=Sharp-Crested Rectangular Weir ( Controls 0.00 cfs)

### Pond 8P: 1

Hydrograph





STEVEN DANZER, PHD & ASSOCIATES LLC

Wetlands & Environmental Consulting

WWW.CTWETLANDSCONSULTING.COM

203 451-8319

WETLAND BOUNDARIES › POND & LAKE MANAGEMENT › CONSTRUCTION FEASIBILITY CONSULTATIONS › ENVIRONMENTAL STUDIES

## Additional Comments

### 417 Main Street, Monroe, CT

Date: February 15, 2022

By: Steven Danzer Ph.D.

- Soil Scientist – Certified Nationally by the Soil Science Society of America (#353463).  
– Registered with the Society of Soil Scientists of Southern New England.
- Senior Professional Wetland Scientist - PWS #1321, Society of Wetland Scientists.
- Arborist - CT DEEP License S-5639; ISA Certified NE-7409A.
  
- Ph.D. - Renewable Natural Resource Studies.

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#### 1. Improvements to the Wetland Restoration and Creation Plan

The mitigation area has doubled since the last plan. The new plan (revised through 2/16/23) indicates 2,360 sf of enhancement/buffer plantings and 1,850 sf of wetland creation, for a total of 4,210 sf of mitigation area. The plant list has been updated accordingly.

The previous plan had 1,000 sf of enhancement/buffer plantings and 990 sf of wetland/creation for a total of 1,990 sf of mitigation area.

#### 2. Impacts due to changes in water level on the wetland/watercourse areas

The projected drawdown will be 8-10 inches. During major events the water level is expected to rise to about current level and then drain quickly. Under existing conditions the plant life within the wetland fringes of the pond is already adaptive to fluctuations in

inundation (ponding) and saturation. The proposed plantings have been similarly selected to establish and thrive under variable conditions. In the past there has been a wide variability in the water level of the pond as evidenced by the abundance of dead trees and snags easily viewed peeking above the water throughout the interior of the pond. The presence of these dead trees suggests that portions of the pond trended more towards a forested swamp in the recent past, attesting to the dynamic nature of this ecosystem.

3. Parking of trucks in the rear of the building.

Regarding staff/commissioner concerns about the food trucks parked in the rear, the vehicles are parked on the existing asphalt, which also provides multiple access ways to the rear of the building. The presence of these vehicles appears to have no additional discernable impact on the wetlands/waterbody. The lawn strip and proposed buffer plantings help filter runoff from the impervious area.

-----  
Thank you for the opportunity to comment.

Respectfully submitted,

Signed,



Steven Danzer Ph.D.

Professional Wetland Scientist, Soil Scientist, Arborist,  
Ph.D. in Renewable Natural Resource Studies





Rev. Kevin Merritt, Senior Pastor  
419-423 Main Street, Monroe CT 06468  
stepneybaptist@gmail.com  
(203) 268-9680

February 6, 2023

Jason Edwards & Associates  
227 Stepney Rd.  
Easton, CT  
06470

Re: Monroe IWC application # IWC-2022-16, File #1421 (417 Main St)

Dear Mr. Edwards,

I am writing to express my unreserved support for the above referenced drainage modification plan for 417 Main St. I write as the longstanding pastor of the church immediately adjacent, Stepney Baptist Church, 419-423 Main St.

I have reviewed the proposed drainage modifications articulated on the site plan presented by your firm. I have also walked the area in question to personally observe the current drainage system in place. From my perspective, this work of enclosing this exposed drainage ditch to extend the existing subterranean drainage system out to the pond is an excellent plan.

I fully support this plan, which includes filling approximately 3700sf of areas considered wetlands at the mouth and along the course of this current open ditch.

Enclosing this drainage ditch and extending the underground system out to the pond will be an enhancement of safety and does not alter the integrity of the pond in any way that I can see or imagine.

My support extends to any variation or modification of the existing plan which may come about, that accomplishes essentially the same goal of allowing for underground seasonal runoff from the pond to be piped to extend the existing underground drainage system.

Sincerely,

A handwritten signature in black ink, appearing to be "Kevin Merritt", written over a long horizontal line.

Rev. Kevin A. Merritt, Senior Pastor

cc: Mr. Ray Ganim, property owner, 417 Main St., Monroe CT 06468

On Tue, Jan 24, 2023 at 2:49 PM Dave Sippin <[dave@sippin.com](mailto:dave@sippin.com)> wrote:

Inland Wetlands Commission

Town of Monroe

Dear Mr. Chairman,

I am an abutting property owner of the Stepney Crossing shopping center at 435 Main Street, Monroe. I have reviewed the above-referenced application to modify the drainage at 417 Main Street. I see no adverse impact to the nearby inland wetlands, and I am in support of this application.

Dave

David B. Sippin

Manager

DG Commercial, LLC



**David B. Sippin | Principal**

234 Main St. Monroe, CT 06468

Direct: 203-880-6783

Mobile: 203-209-6698

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**From:** [Tim Bartlett](#)  
**To:** [Ray Ganim](#)  
**Cc:** [Jason Edwards](#)  
**Subject:** RE: Monroe IWC Application #IWC-2022-16, File #1421 (417 Main Street)  
**Date:** Saturday, February 4, 2023 8:21:00 AM

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Good Morning Ray –

I forwarded the information you provided and spoken with Dave Stevenson, our CEO & President. Based on what we have seen and read and the conversations you and I have had, we support your project and look forward to the progress. Please keep us in the loop so we may be further engaged should the situation change.

Thanks

Tim

**From:** Ray Ganim <ray@ganimlaw.com>  
**Sent:** Wednesday, February 1, 2023 12:51 PM  
**To:** Tim Bartlett <tbartlett@cccymca.org>  
**Cc:** Jason Edwards <jason@jedwardsassoc.com>  
**Subject:** Re: Monroe IWC Application #IWC-2022-16, File #1421 (417 Main Street)

Tim-

It was nice speaking with you today. Attached is a copy of the plan we submitted for approval to the Monroe Inland Wetlands Commission for piping a small section of an open ditch. Based on comments from the Town's consultant, the plan may change in some respects to accomplish essentially the same goal of piping the open ditch. One of the issues being dealt with currently is the reported increased water level in the pond which may be due to some maintenance needed to the existing drainage system as water flows from the pond out to Main Street in the wet seasons of the year. Just so you are aware, some details of the plan may change based on discussions with the Town's consultant and the engineer I have engaged on the project.

The next meeting of the Inland Wetlands Commission is at 7PM on Wednesday, February 8, 2023. I am looking to have the support of the adjoining property owners to this plan which includes the YMCA. This support may be in the form of a reply email to me stating that the YMCA has reviewed the application and discussed the matter with me and that the YMCA supports the approval of the application as may be modified in order to reduce the heightened water levels in the pond that might be determined to exist due to any impediments to the water flow.

Thanks again and feel free to call with any questions.

**Raymond W. Ganim, Esq.**



2620 Nichols Avenue

Stratford, CT 06614

**Tel (203) 377-7700**

Fax (203) 377-7900

e-mail: [ray@ganimlaw.com](mailto:ray@ganimlaw.com)

Review Me at <https://t.avvo.com/x0qn>

Visit us at [www.ganimlaw.com](http://www.ganimlaw.com)

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March 1, 2023

Below area responses to the comments received from the Monroe Department of Public Works on 01.23.23. Responses from JEA are in [blue](#)

1. Correct the references in the title block on Drawings 2.0 through 5.0 that refer to the property address as “407 Main Street”.

[This has been addressed](#)

2. Add north arrows to Drawings 2.0 through 5.0.

[This has been addressed](#)

3. The Stormwater Operations and Maintenance Plan on Drawing 4.1 is generic and refers to elements such as infiltration galleries, sediment basin, etc., that are not part of the design. Revise to be project-specific. Include measures for maintenance of headwall maintenance of enhancement measures.

[This has been addressed](#)

4. Alternatives to meet the “feasible and prudent” test required by the Inland Wetlands regulations were not found. Please provide a narrative and supporting diagrams. The proposed design is likely the most feasible and prudent alternative, but the record on other alternatives considered, including no-build, should be clear.

[The current plan submission is significantly different and represents an alternative the original submission](#)

5. The project will require water handling to bypass watercourse flow through the project area. Show water handling measures on the Sediment and Erosion Control Plan, and include the footprint required for the construction and placement of these measures as impacts if they have not been accounted for in the initial computation of disturbance areas. We suggest that the applicant’s engineer follow the guidance in Chapter 6.F of the Connecticut Department of Transportation’s Drainage Manual for the design of temporary hydraulic facilities. The guidance will establish the “Design Risk” for the project, which can then be used to determine a design storm frequency for water handling measures, such as temporary cofferdams. Note that any cofferdam will need at least one foot of freeboard above the design elevation. Revise the construction sequence on Drawing 4.1 accordingly.

[This has been addressed. The new piping has been shifted to allow the existing IWV to continue to function during construction. Therefore the originally calculated disturbance area is accurate.](#)

6. Revisions are required to the grading east of the existing residential structure:
  - a. Provide contour labeling for existing conditions contours.
  - b. A low point encircled by the 350 contour will be created with no means for positive drainage.
  - c. The grading shown creates a 2H:1V slope within the footprint of the west leg of the existing gravel driveway. However, it appears as if the west leg of the driveway is proposed to be abandoned. Provide clarifying notes and/or hatching regarding segments of driveway to be abandoned, to remain, or be relocated.
  - d. Provide gravel driveway typical section detail.
  - e. If the area north of the proposed residential structure is raised in elevation, will the existing stairs at the end of the walkway and the walkway itself get removed or otherwise reconfigured? Show final disposition and limits on plans.

These have been addressed

7. The storm drain pipe from the parking lot entering proposed Storm Manhole 1 connects at an adverse angle, oriented in opposition to the flow from the watercourse. Consider moving Storm Manhole 1 approximate 16 to 17 feet south so that a new pipe can be connected from the existing storm drain manhole (TF 350.72) to relocated Storm Manhole 1 at a more favorable 90 degree angle?

The manhole has been relocated to better align with existing piping

8. The portion of the existing storm drainage system on the south curb line of the existing parking lot, from the double catch basin (TF 350.38) to the manhole (TF 350.72) is shown with portions inaccessible. Investigate how the manhole between the two structures can be made accessible. The length of the system should be cleaned as a condition of approval.

These structures are to be cleaned and inspected as part

9. Provide details for the proposed headwall. The plans graphically show a headwall and a flared end. A headwall is to be used to prevent the stream from undercutting.

The headwall is no longer planned. A catch basin frame is to be used as an outlet control structure.

10. At the eastern end of the relocation of the existing gravel driveway, adjust the tie in location to avoid SNET Pole #2431 such that it is removed from the footprint of the proposed driveway relocation.

This has been addressed

11. Adjust the alignment of the pipe run between the upstream headwall and proposed Storm Manhole 1 to avoid conflict with SNET Pole #2432.

This has been addressed

12. There is an existing 6" PVC roof leader that discharges from the southeast corner of the building into the watercourse. How will this roof leader be picked up into the new culvert system?

This will be routed to MH1. This is now depicted on the plan

13. The proposed silt fence crosses the existing and proposed gravel driveway multiple times. If the existing home will be occupied during construction operations, the silt fence should be relocated to accommodate access to the structure.

[This has been addressed](#)

14. Will the two existing trees within the wetland area and the tree near the gravel driveway relocation be removed?

[These will be removed](#)

15. Prior to construction, we suggest the excavation of test pits above the areas where the subsurface sewage disposal system force main and electric conduit cross the proposed 24" pipe to confirm that these utilities are not in conflict with the proposed culvert alignment.

[We have added notes requiring test pits](#)

16. The applicant is advised to contact the U.S. Army Corps of Engineers and the Connecticut Department of Energy and Environmental Protection, as the work will likely require authorization under the USACE Connecticut General Permit.

[This will be done if approval is granted by the commission](#)

A completed bond estimate was not found in the application materials.

[A bond estimate will be submitted](#)

#### The following is a response to Town Staff and Commission comments from previous meetings

1. Please have the wetland scientist assess that impact of the proposed water level changes on the health and viability of the wetland and watercourse at the rear of the property.  
[A response from Steven Danzer has been provided.](#)
2. Submit a revised site plan with the URA clearly shown in red  
[This has been addressed.](#)
3. A storm water runoff protection plan for the rear of the site, and a stipulation that no trucks be parked in the back of the building.  
[Runoff protection is incorporated as part of the landscape design. See response number 3 in comments provided by Steven Danzer.](#)
4. Confirm the 10" pipe elevation, and is the sewer forced main on top or beneath it?  
[The force main is beneath the pipe.](#)

Staff will also be recommending the following conditions as part of the draft resolution:

1. That there be a 1 or 2 year planting plan reassessment conducted to confirm success and viability, and that any invasive species have not re-established.
2. A possible limit of lawn area included on the plans, with "No mow" signs posted.
3. Proposed locations of snow piles be shown on the approved plan to ensure protection of habitat from damage and salt overload.
4. "Regulated Area" signage be posted.

[The applicant requests that these items be addressed as conditions if the application is approved. Locations of signage can be worked out with Staff.](#)